ME340B – Elasticity of Microscopic Structures – Wei Cai – Stanford University – Winter 2004 Problem Set 2 Solution Elasticity in one and two dimensions

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Problem 2.1 (10') Elastic constants.

The elastic stiffness tensor for the isotropic medium is $C_{ijkl} = \lambda \delta_{ij} \delta_{kl} + \mu (\delta_{ik} \delta_{jl} + \delta_{il} \delta_{jk})$. Determine the compliance tensor, S_{ijkl} , which is the inverse of C_{ijkl} , i.e.,

$$C_{ijkl}S_{klmn} = \frac{1}{2}(\delta_{im}\delta_{jn} + \delta_{in}\delta_{jm}) \tag{1}$$

Solution:

$$C_{ijkl}S_{klmn} = [\lambda\delta_{ij}\delta_{kl} + \mu(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk})] [\alpha\delta_{ij}\delta_{kl} + \beta(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk})]$$

$$= (3\lambda\alpha + 2\mu\alpha + 2\beta\lambda)\delta_{ij}\delta_{mn} + 2\beta\mu(\delta_{im}\delta_{jn} + \delta_{in}\delta_{jm})$$

$$= \frac{1}{2}(\delta_{im}\delta_{jn} + \delta_{in}\delta_{jm})$$

Thus

$$2\beta\mu = \frac{1}{2}$$
$$\beta = \frac{1}{4\mu}$$

and

$$\begin{aligned} &3\lambda\alpha + 2\mu\alpha + 2\beta\lambda = 0\\ &3\lambda\alpha + 2\mu\alpha + \frac{\lambda}{2\mu} = 0\\ &(3\lambda + 2\mu)\alpha = -\frac{1}{2}\frac{\lambda}{\mu}\\ &\alpha = -\frac{1}{2}\frac{\lambda}{\mu(3\lambda + 2\mu)}\\ &S_{ijkl} = -\frac{1}{2}\frac{\lambda}{\mu(3\lambda + 2\mu)}\delta_{ij}\delta_{kl} + \frac{1}{4\mu}(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk}) \end{aligned}$$

Explicit expressions of C_{ijkl} and S_{ijkl}

Let us write out the various terms of C_{ijkl} and S_{ijkl} explicitly in isotropic elasticity. There are only three different terms in C_{ijkl} : C_{1111} , C_{1122} and C_{1212} . Other terms can be obtained by symmetry, e.g. $C_{2233} = C_{1122}$. In contracted notation, these three terms are written as C_{11} , C_{12} , C_{44} .

$$C_{1111} = C_{11} = \lambda + 2\mu \tag{2}$$

$$C_{1122} = C_{12} = \lambda \tag{3}$$

$$C_{1212} = C_{44} = \mu \tag{4}$$

Therefore, an isotropic elastic medium has the property that

$$C_{11} = C_{12} + 2C_{44} \tag{5}$$

Hence the anisotropic factor,

$$A \equiv \frac{2C_{44}}{C_{11} - C_{12}} \tag{6}$$

equals to one for an isotropic medium. This is of course not the case for an anisotropic medium. For crystals with cubic symmetry, the elastic constants, C_{11} , C_{12} and C_{44} are independent of each other.

$$S_{1111} = -\frac{\lambda}{2\mu(3\lambda+2\mu)} + \frac{1}{2\mu} = \frac{\lambda+\mu}{\mu(3\lambda+2\mu)} \equiv \frac{1}{E}$$
(7)

$$S_{1122} = -\frac{\lambda}{2\mu(3\lambda + 2\mu)} \tag{8}$$

$$S_{1212} = \frac{1}{4\mu}$$
 (9)

where E is called the Young's modulus. Notice that $\lambda = 2\mu\nu/(1-2\nu)$, thus

$$S_{1111} = \frac{1}{E} = \frac{1}{2\mu(1+\nu)} \tag{10}$$

$$S_{1122} = -\frac{\nu}{E}$$
 (11)

$$S_{1212} = \frac{1+\nu}{2E}$$
(12)

These results are the bases of Problem 2.3(b).

Problem 2.2 (10') 1D elasticity.

Determine the displacement, strain and stress field of a long rod of length L standing vertically in a gravitational field g. Assume the rod is an isotropic elastic medium with shear modulus μ and Poisson's ratio ν .

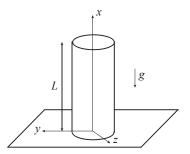


Figure 1: A rod of length L standing vertically in a gravitational field g.

Solution:

Choose the coordinate system such that x-axis goes along the axis of the rod pointing up with the origin at the bottom of the rod. The equation of equilibrium is,

$$\sigma_{xx,x} + b_x = 0 \tag{13}$$

while all the other stress components are zero, i.e.,

$$\sigma_{yy} = \sigma_{zz} = \sigma_{xy} = \sigma_{yz} = \sigma_{zx} = 0 \tag{14}$$

The boundary condition for Eq. (13) is such that $\sigma_{xx} = 0$ at x = L. Because $b_x = -\rho g$ (ρ is the density of the rod), the solution is

$$\sigma_{xx} = \rho g(x - L) \tag{15}$$

From Problem 2.1, noting that for Hooke's Law we have

$$e_{xx} = S_{1111}\sigma_{xx} = \frac{\sigma_{xx}}{E} = \frac{\rho g}{E}(x-L) \tag{16}$$

where $E = 2\mu(1 + \nu)$ is the Young's modulus. Similarly,

$$e_{yy} = S_{1122}\sigma_{xx} = -\frac{\nu\rho g}{E}(x-L)$$
 (17)

$$e_{zz} = S_{1122}\sigma_{xx} = -\frac{\nu\rho g}{E}(x-L)$$
 (18)

$$e_{xy} = e_{yz} = e_{zx} = 0$$
 (19)

Therefore,

$$u_{x,x} = \frac{\rho g}{E}(x-L) \tag{20}$$

$$u_{y,y} = -\frac{\nu\rho g}{E}(x-L) \tag{21}$$

$$u_{z,z} = -\frac{\nu\rho g}{E}(x-L) \tag{22}$$

$$u_{x,y} = -u_{y,x} \tag{23}$$

$$u_{x,z} = -u_{z,x} \tag{24}$$

$$u_{y,z} = -u_{z,y} \tag{25}$$

We wish to apply the boundary condition of $u_x = 0$ at x = 0. The following solution satisfies the boundary condition of $u_x = 0$ at x = y = z = 0 (i.e. boundary condition only imposed at a single point at the bottom of the rod),

$$u_x = \frac{\rho g}{E} \left[\frac{1}{2} x^2 - Lx + \frac{\nu}{2} (y^2 + z^2) \right]$$
(26)

$$u_y = -\frac{\nu \rho g}{E} (x - L) y \tag{27}$$

$$u_z = -\frac{\nu \rho g}{E} (x - L)z \tag{28}$$

The above solution does not satisfy the boundary condition at the entire plane of x = 0. Therefore solution is not valid near the end of the rod. (The rod is now standing on a quadratic surface.) To fully account for the end effect of a flat surface, the stress will no longer be a simple one-dimensional function as given by Eq. (15).

Problem 2.3 (10') 2D elaticity.

Lets look at equilibrium in 2-D elasticity using x-y cartesian coordinates under zero body force. Assume the 2-d body is in a state of plane stress, i.e.,

$$\sigma_{zx} = \sigma_{zy} = \sigma_{zz} = 0$$

which corresponds to a free standing thin film. The equilibrium equations reduce to

$$\sigma_{xx,x} + \sigma_{yx,y} = 0 \tag{29}$$

$$\sigma_{yy,y} + \sigma_{xy,x} = 0 \tag{30}$$

And the compatability equations reduce to

$$e_{xx,yy} - 2e_{xy,xy} + e_{yy,xx} = 0 \tag{31}$$

One popular method to solve such problems is to introduce the Airy's stress function ϕ such that,

$$\sigma_{xx} = \phi_{,yy} \tag{32}$$

$$\sigma_{yy} = \phi_{,xx} \tag{33}$$

$$\sigma_{xy} = -\phi_{,xy} \tag{34}$$

(a) Show that this particular choice of stress function automatically satisfies equilibrium.

Solution:

$$\sigma_{xx,x} + \sigma_{yx,y} = \phi_{,yyx} + (-\phi_{,xyy}) = 0$$

$$\sigma_{yy,y} + \sigma_{xy,x} = \phi_{,xxy} + (-\phi_{,yxy}) = 0$$

(b) Assuming that Hooke's Law is of the form

$$e_{xx} = \frac{\sigma_{xx}}{E} - \frac{\nu \sigma_{yy}}{E} \tag{35}$$

$$e_{yy} = \frac{\sigma_{yy}}{E} - \frac{\nu \sigma_{xx}}{E} \tag{36}$$

$$e_{xy} = \frac{\sigma_{xy}(1+\nu)}{E} \tag{37}$$

show that the compatability equation reduces to

$$\phi_{,xxxx} + 2\phi_{,xxyy} + \phi_{,yyyy} = 0 \tag{38}$$

Solution:

Starting from the compatibility equation,

$$e_{xx,yy} - 2e_{xy,xy} + e_{yy,xx} = 0$$

plug in the Hooke's law,

$$\frac{1}{E} \left(\sigma_{xx,yy} - \nu \sigma_{yy,yy} + \sigma_{yy,xx} - \nu \sigma_{xx,xx} \right) - 2 \frac{(1+\nu)}{E} \sigma_{xy,xy} = 0$$

$$\phi_{,yyyy} - \nu \phi_{,xxyy} + \phi_{,xxxx} - \nu \phi_{,yyxx} + 2(1+\nu)\phi_{,xyxy} = 0$$

$$\phi_{,xxxx} + 2\phi_{,xyxy} + \phi_{,yyyy} = 0$$

This is the biharmonic equation, which is often written as $\nabla^4 \phi = 0$.

(c) What is the relation between E and the shear modulus μ and Poisson's ration ν ?

Solution:

 $E = 2\mu(1+\nu)$

see box in Problem 2.1.

(d) Note that the solution of Eq. (38) does not depend on elastic constants. Let's use this solution to solve a very simple stress problem. Consider a square of length a under hydrostatic

pressure P. What are the stress components inside the box? (guess!) What is the stress function ϕ ?

Solution:

The box is under uniform stress,

$$\sigma_{xx} = -P$$

$$\sigma_{yy} = -P$$

$$\sigma_{xy} = 0$$

$$\phi = -\frac{1}{2}Py^2 - \frac{1}{2}Px^2$$

The strain tensor is,

$$e_{xx} = e_{yy} = -P\frac{1-\nu}{E}$$
$$e_{xy} = 0$$

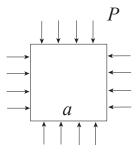


Figure 2: A square of length a under hydrostatic pressure P.